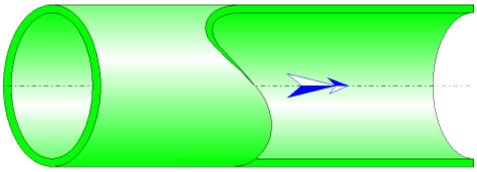
Straight Pipe
Circular Cross-Section and Nonuniform Roughness Walls
(IDELCHIK)



Model description:

This model of component calculates the major head loss (pressure drop) of a horizontal straight pipe of circular and constant cross-section.
In addition, the flow is assumed fully developed and stabilized.

The head loss is due to the friction of the fluid on the inner walls of the piping and is calculated with the Darcy formula. The roughness of the inner walls of the pipe is supposed nonuniform (commercial pipe).

Darcy friction factor is determined:

- for laminar flow regime by the law of Hagen-Poiseuille (independent of the value of relative roughness),
- for turbulent flow regime by the implicit Colebrook-White equation (dependent of the value of relative roughness),
- for critical flow regime by interpolation between friction factors of laminar and turbulent flow.

Model formulation:

Hydraulic diameter (m):

$$D_h = D_0$$

Cross-section area (m²):

$$F_0 = \pi \cdot \frac{D_0{}^2}{4}$$

Mean velocity (m/s):

$$w_0 = \frac{Q}{F_0}$$

Mass flow rate (kg/s):

$$G = Q \cdot \rho$$

Fluid volume in the pipe (m^3):

$$V = F_0 \cdot l$$

Fluid mass in the pipe (kg):

$$M = V \cdot \rho$$

Reynolds number:

$$\mathrm{Re} = \frac{w_0 \cdot D_h}{\nu}$$

Relative roughness:

$$\overline{\Delta} = \frac{\Delta}{D_h}$$

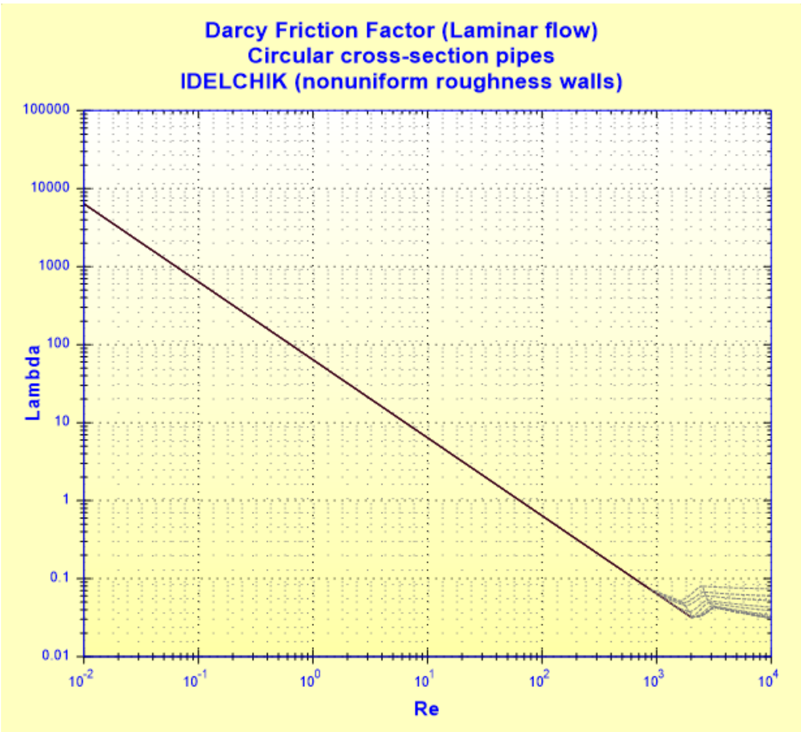
Darcy friction factor:

- laminar flow regime ($\mathrm{Re} \le \mathrm{Re}_0$):

  Hagen-Poiseuille law

$$\lambda = \frac{64}{\mathrm{Re}}$$

([1] diagram 2.1)



- turbulent flow regime – transition region and complete turbulence region ($\mathrm{Re} \ge \mathrm{Re}_2$):

  Colebrook-White equation

$$\lambda = \frac{1}{\left[2 \cdot \log\left(\frac{2.51}{Re \cdot \sqrt{\lambda}} + \frac{\overline{\Delta}}{3.7}\right)\right]^2}$$

([1] diagram 2.4)

Reynolds number at which pipe cease to be hydraulically smooth:

$$Re'_{lim} = \frac{15}{\overline{\Delta}}$$

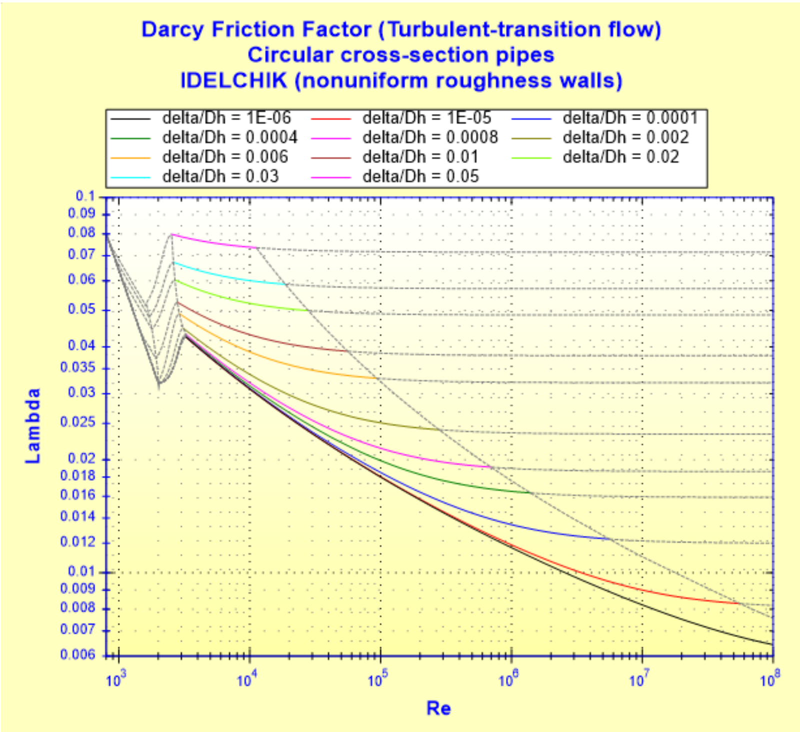
([1] §2.23)

Reynolds number corresponding to the beginning of complete turbulence:

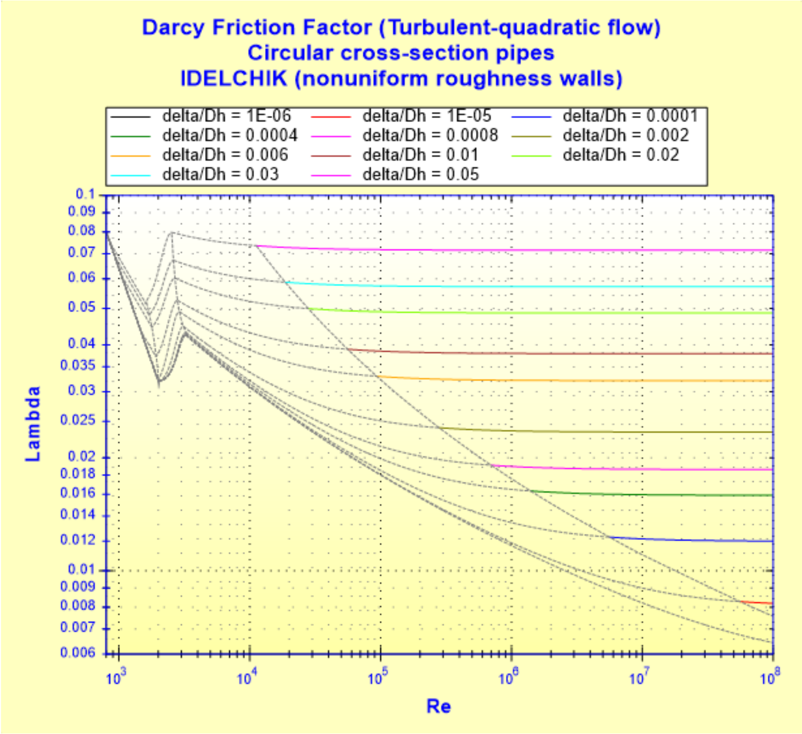
$$Re''_{lim} = \frac{560}{\overline{\Delta}}$$

([1] diagram 2.4)

Transition region



Complete turbulence region



■ critical flow regime ($Re_0 < Re < Re_2$):

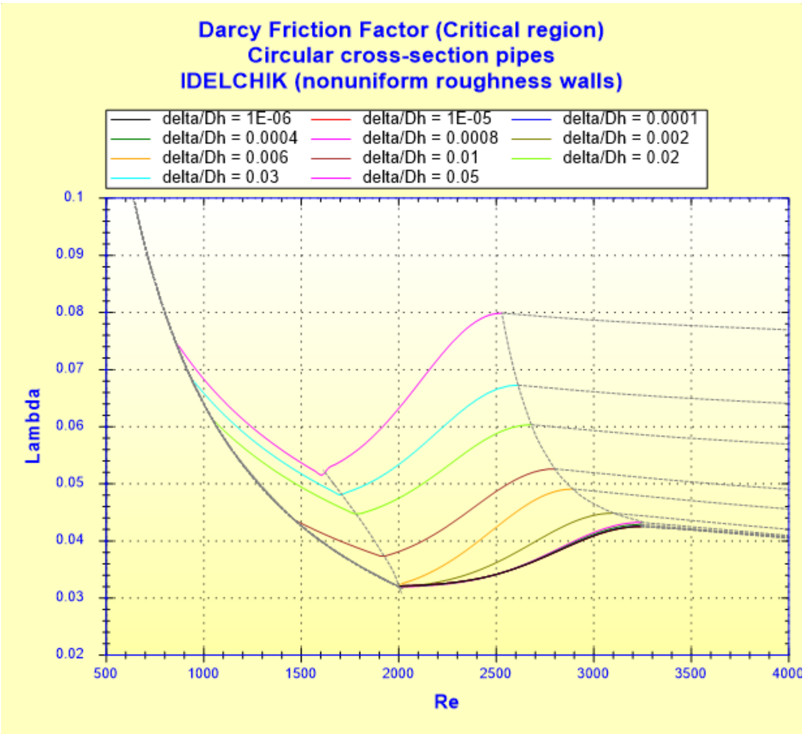
$$\lambda = f(\mathrm{Re}, \bar{\Delta})$$ ([1] diagram 2.3)

Reynolds number of start of critical zone:

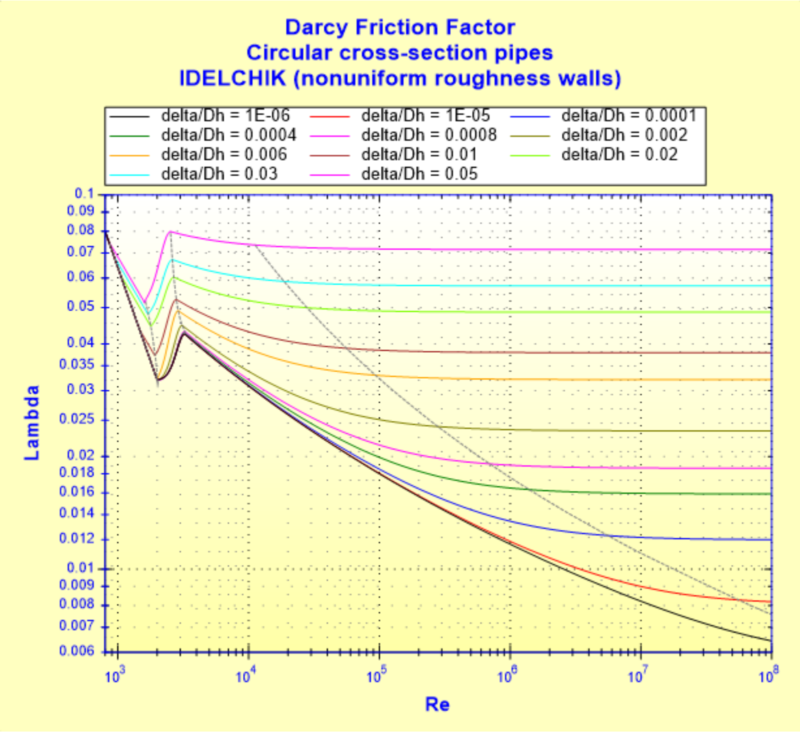
$$\mathrm{Re}_0 = 754 \exp\left(\frac{0.0065}{\bar{\Delta}}\right)$$ ([1] §2.21)

Reynolds number at end of critical zone:

$$\mathrm{Re}_2 = 2090 \left(\frac{1}{\bar{\Delta}}\right)^{0.0635}$$ ([1] §2.22)



■ all flow regimes:



Pressure loss coefficient (based on the mean pipe velocity):

$$\zeta = \lambda \cdot \frac{l}{D_h}$$

([1] equation 2-2)

Total pressure loss (Pa):

$$\Delta P = \zeta \cdot \frac{\rho \cdot w_0^{\ 2}}{2}$$

([1] equation 2-2)

Total head loss of fluid (m):

$$\Delta H = \zeta \cdot \frac{w_0^{\ 2}}{2 \cdot g}$$

Hydraulic power loss (W):

$$Wh = \Delta P \cdot Q$$

Symbols, Definitions, SI Units:

D_h	Hydraulic diameter (m)
D_0	Internal diameter (m)
F_0	Cross-sectional area (m^2)
Q	Volume flow rate (m^3/s)
G	Mass flow rate (kg/s)
w_0	Mean velocity (m/s)
l	Pipe length (m)
V	Fluid volume in the pipe (m^3)
M	Fluid mass in the pipe (kg)
Re	Reynolds number ()
Re_0	Reynolds number of start of critical zone ()

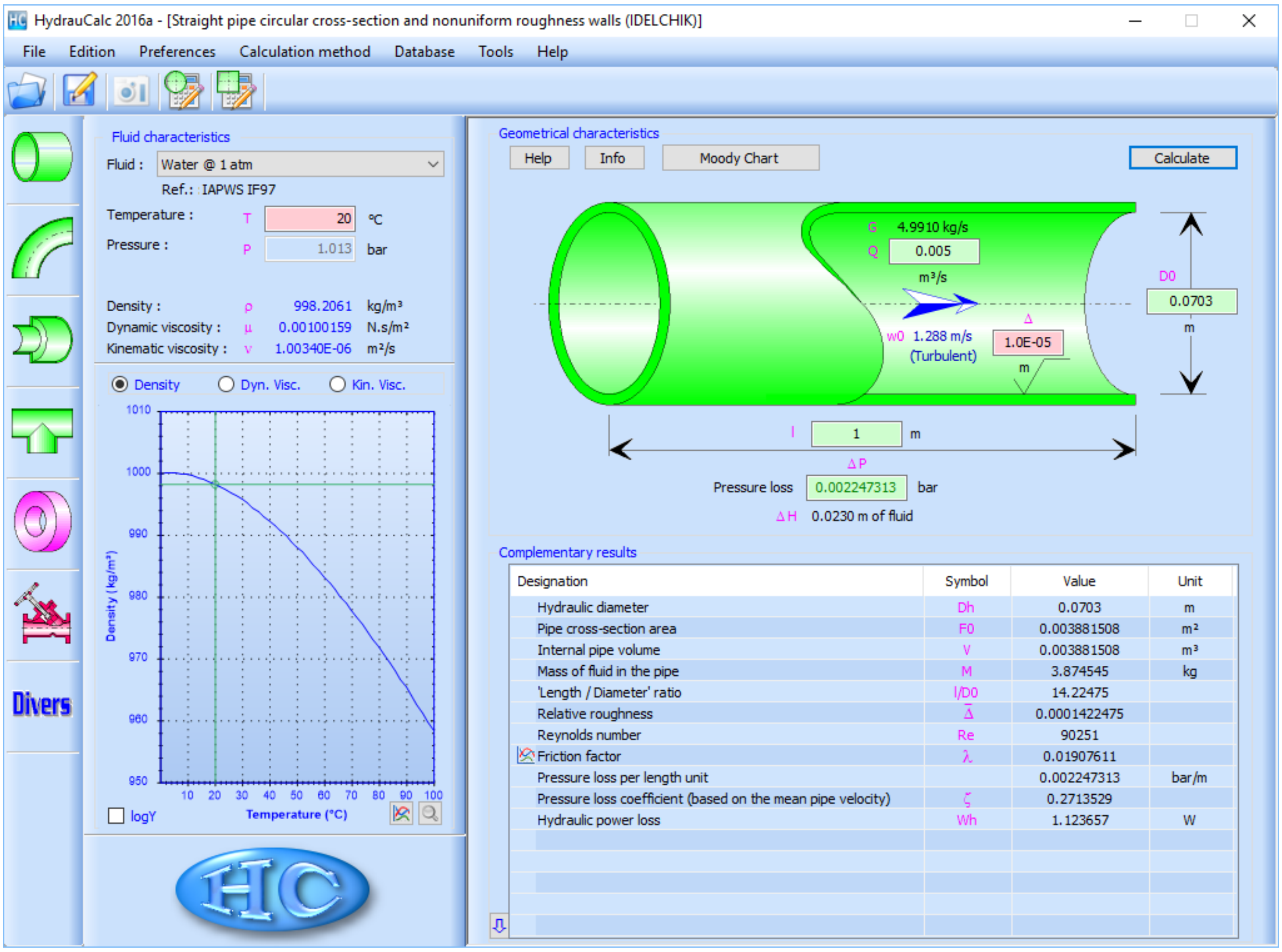
Re_2 Reynolds number at end of critical zone ()
Re'_{lim} Limiting Reynolds number for hydraulically smooth law ()
Re''_{lim} Limiting Reynolds number for quadratic law ()
Δ Absolute roughness of walls (m)
$\overline{\Delta}$ Relative roughness of walls ()
λ Darcy friction factor ()
ζ Pressure loss coefficient (based on the mean pipe velocity) ()
ΔP Total pressure loss (Pa)
ΔH Total head loss of fluid (m)
Wh Hydraulic power loss (W)

ρ Fluid density (kg/m^3)
ν Fluid kinematic viscosity (m^2/s)
g Gravitational acceleration (m/s^2)

Validity range:

- any flow regime: laminar, critical and turbulent ($Re \leq 10^8$)
- $\overline{\Delta} \leq 0.05$
- stabilized flow

Example of input data and results:



References:

[1] Handbook of Hydraulic Resistance, 3rd Edition, I.E. Idelchik (2008)

HydrauCalc
Edition: February 2018